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ACI Workshop on Advances in Service Life Models that Consider Multiple Sources of Deterioration Atlanta, Georgia, April 24,2007

A Probabilistic Cumulative Damage Model for Service Life Prediction of Concrete Structures

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- Introduction
- Performance of Concrete Structures
- Probabilistic Cumulative Damage Models
- Example: Concrete Bridge Decks
- Conclusions

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Introduction

- Concrete structures: multiple components
 - aging and requiring maintenance (e.g. bridges)
 - reduced strength, serviceability, functionality
 - several failure mechanisms with interaction- very complex to model

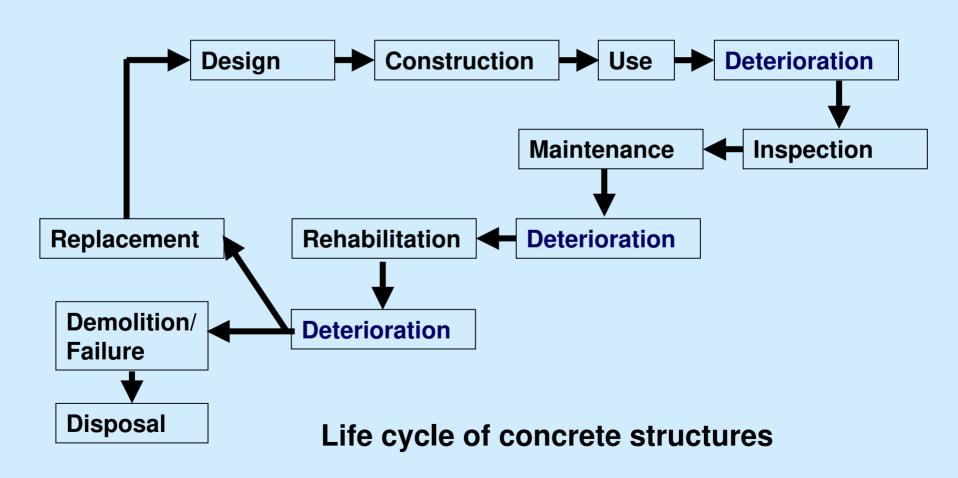
- Multiple deterioration mechanisms & failure modes
 - corrosion, freezing & thawing, alkali-aggregate reaction
 - fatigue, overstress (overload and/or under-strength)
 - partial/total collapse (bending, shear, punching shear,..)
 - loss of serviceability (excessive deformation, cracking)

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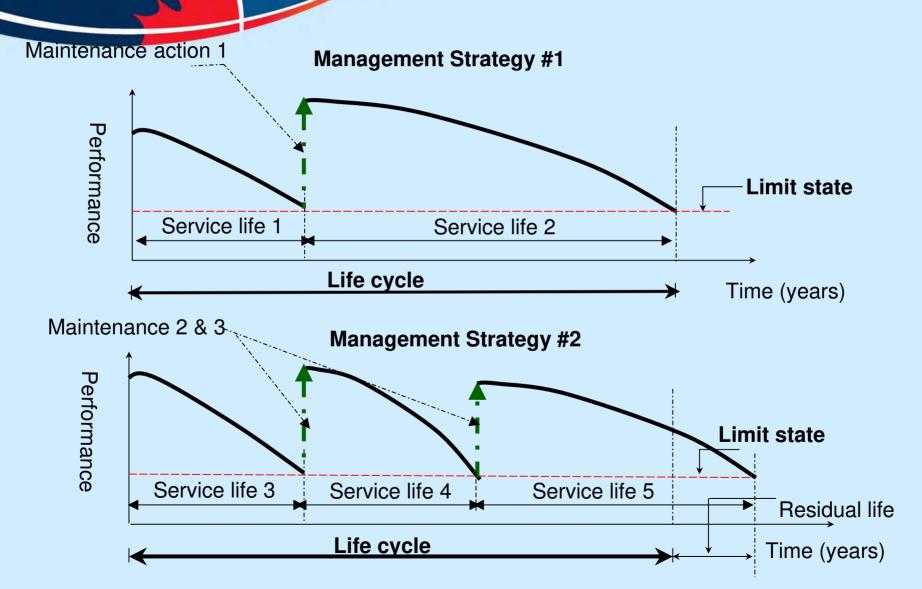
Introduction

- Causes of deterioration/failure
 - Increased loads
 - Aggressive environment
 - Reduced strength and/or stiffness
 - Inadequate design, construction, and maintenance
- Consequences of deterioration/failure
 - reduced safety, serviceability and service life
 - reduced level of service or functionality
 - increased risk of fatalities/injuries
 - increased maintenance and user costs
 - increased environmental impact





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- Need service life models to:
 - Predict times it take to reach relevant limit states for different design and maintenance options
 - Ensure safety, serviceability & functionality
 - Evaluate life cycle costs
 - Optimize design, inspection and maintenance

Definitions:

- Service life: time to reach a limit state: time to failure
- Life cycle: time period for analysis
- Residual life remaining life at end of life cycle



- Benefits of life cycle performance modeling
 - identify critical structures/components
 - determine optimal times for maintenance
 - identify optimal maintenance strategies
 - extend service life of structures
 - minimize life cycle costs
 - determine required funding over life cycle
 - optimize the design of new structures



- Prediction of life cycle performance of concrete structures: complex problem
 - structures deteriorate at different rates
 - uncertainty in loading, material properties, deterioration mechanisms, structural response, etc.
 - lack of reliable performance data
 - many owners/engineers use qualitative indicators to assess structural performance



- Existing service life models
 - empirical models (accelerated life aging)
 - heuristic models
 - factorial models
 - logistic models
 - mechanistic models
 - cumulative damage models
 - shock models
 - probabilistic models



- Existing probabilistic service life models
 - statistical lifetime models
 - first or second order reliability models
 - Monte-Carlo simulation-based models
 - outcrossing models
 - stochastic finite element models
 - probabilistic cumulative damage models



- Progressive accumulation of irreversible damage
 - cracks, spalls, delamination due to corrosion
 - cracks due to load, imposed deformations, freeze-thaw cycles, alkali-aggregate reactivity
 - inelastic deformations, fatigue, etc.
- Consideration of uncertainty
- Compatible with existing inspection methodologies of different structural concrete systems



- PCD model 1
 - deterministic law for damage accumulation
 - parameters as random variables or processes
 - time-dependent accumulated damage
- PCD model 2- Bogdanoff's model
 - evolutionary probabilistic model from the start
 - probabilistic distribution of accumulated damage



- Bogdanoff's PCD models
 - based on Markovian processes: simple and practical stochastic processes
 - require limited amount of data
 - models can be easily updated with new data
 - can model impact of maintenance



- Probabilistic distribution of damage:
 - depends on "duty" or "stress" cycle
 - depends on damage accumulated at start of stress cycle
 - does not depend on how the damage was accumulated up to the start of stress cycle
 - damage accumulation: time-increasing and progressive (no "shock" type of damage)



- Probabilistic distribution of damage
 - high noise-to-signal ratios: large fluctuations from mean response
 - uncertainty in initial condition
 - uncertainty in damage accumulation



Deterioration of concrete structures modeled as a stochastic process

- Probabilistic distribution of damage
 - damage discretized into set of states: 1,2,...,b
 - b= absorbing or "failure" state
 - future state is independent of past states given present state ("memoryless" process)

$$P(S_{t+1}=S_{t+1} | S_t=S_t, S_{t-1}=S_{t-1}, .., S_0=S_0)=P(S_{t+1}=S_{t+1} | S_t=S_t)$$

- Probabilistic damage distribution defined by:
 - transition probability matrix for each stress cycle

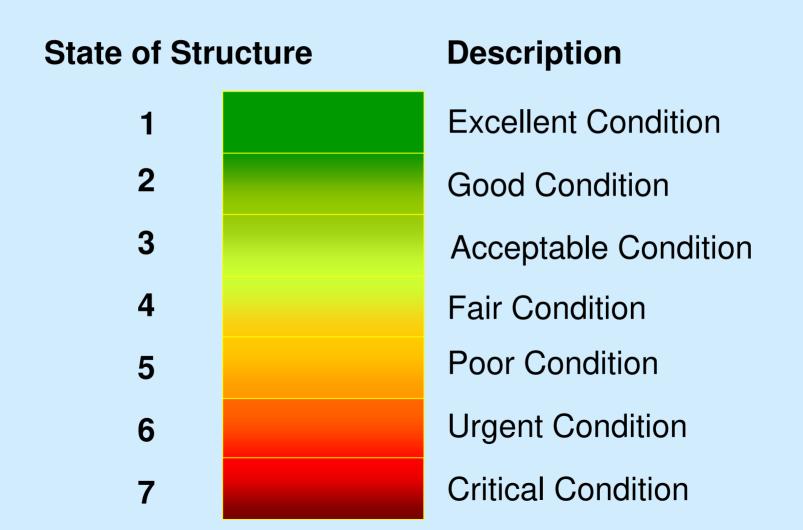
$$P=\{P_{ik}\}$$
: transition matrix $j=1,2,...n$; $k=1,2,...,n$

Pjk= probability of damage being in state k after a stress cycle given it was in state j at start of stress cycle

– initial damage at t=0:

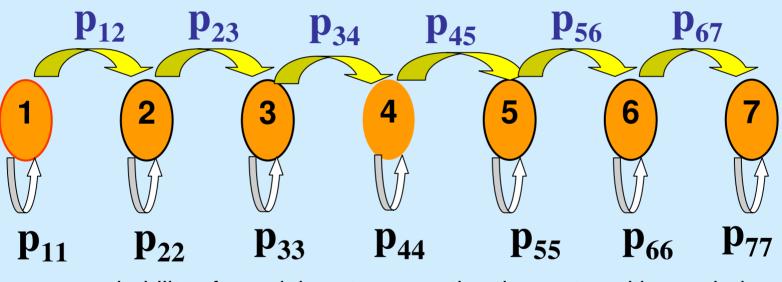
$$\mathbf{p}_0 = [\mathbf{p}_0(1) \ \mathbf{p}_0(2)..... \ \mathbf{p}_0(b)]$$
 =vector of initial state of damage







probability of deteriorating by one rating in one transition period

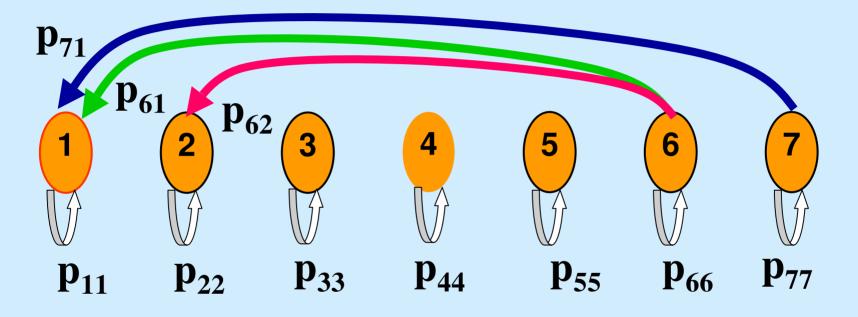


probability of remaining at same rating in one transition period

Unit-Jump Markov Chain Deterioration Model

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Probabilistic Cumulative Damage Models



p_{nm}= probability of upgrading from state n to state m within one transition

Modeling Impact of Maintenance on Performance

Prediction of time-dependent damage

$$p_t = p_0 P_1 P_2 P_t$$

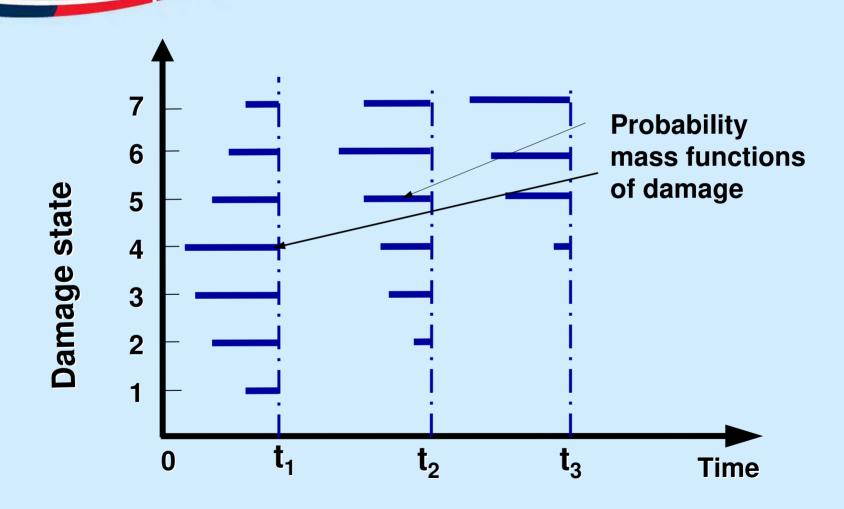
p_t = vector of state of damage at time t

 P_i = transition matrix for j^{th} stress cycle

• If stress cycles are constant over life of structure

$$p_t = P_0 P^t$$

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- Cumulative distribution function of damage
 - D(t)= state of damage at time t

$$F_{Dt}(j) = P[D(t) \le j] = \Sigma_k p_t(k) \quad k=1,..., j$$

Expected damage

$$E[D(t)] = \sum_{k} k p_{t}(k)$$
 $k=1,..., b$

• Service life:T=Time to absorption (i.e. reaching state b)

cdf:
$$F_T(t) = P (T \le t) = p_t(b)$$
 $t=1,2,...,n$

mean T:
$$E(T) = \Sigma_t [1 - F_T(t)]$$



- Model development
 - Historical field performance data
 - Bayesian updating with additional data
 - Explanatory variables
 - Age
 - Structural system type
 - Environment
 - Loading
 - Quality of design / protection/ construction
 - Maintenance option



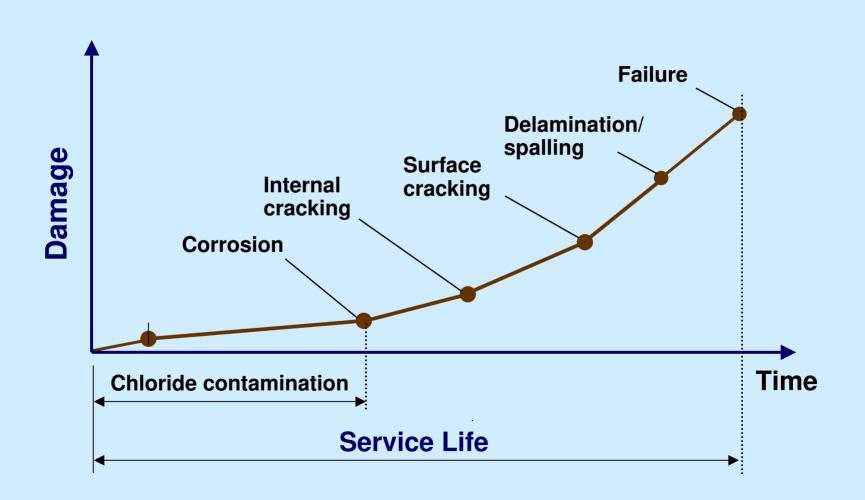
Example: Concrete BridgeDecks

- Reinforced concrete bridge decks
 - Most deteriorated bridge elements
 - 1/3 to 1/2 projected bridge maintenance costs

- Development of PCD model for bridge decks
 - historical field performance data (min. 2 points in time)
 - explanatory variables
 - highway class
 - environmental exposure
 - average daily traffic
 - average daily truck traffic



Example: Concrete BridgeDecks



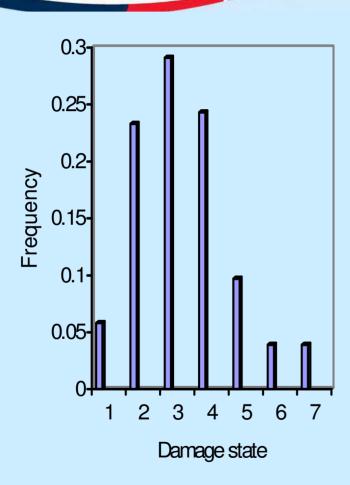
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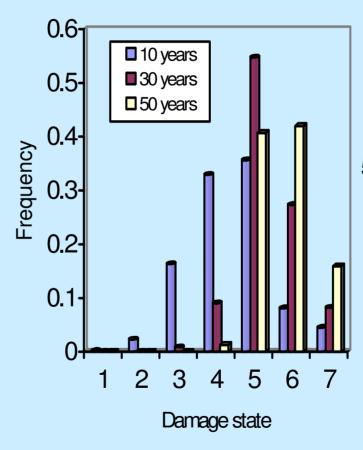
Example: Concrete BridgeDecks

P =	0.70	0.30	0	0	0	0	0
	0	0.75	0.25	0	0	0	0
	0	0	0.85	0.15	0	0	0
	0	0	0	0.90	0.10	0	0
	0	0	0	0	0.98	0.02	0
	0	0	0	0	0	0.98	0.02
	0	0	0	0	0	0	1.00



Example: Concrete BridgeDecks





Expected damage 3.36 @ 0 5.33 @ 30 years

Current condition

Conditions after 10, 30, 50 years

Conclusions

- Service life of concrete structures can be predicted using probabilistic cumulative damage (PCD) models
- PCD models capture time-dependence and uncertainty of performance of concrete structures
- PCD models provide effective decision support for design and maintenance of concrete structures
- PCD models are practical and require limited data
- PCD models can also be used for a large group of structures

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